

Measurement of Apparent Activation Energy of Portland Cement Based Materials

Christopher C. Ferraro, Mang Tia,
Department of Civil and Coastal Engineering, University of Florida

ABSTRACT

The apparent activation energy of Portland cement based materials is a critical parameter for the calculation of maturity and equivalent age characteristics of concrete. The traditional, standardized method (ASTM C1074) utilizes the monitoring of temperature-time history and the compressive strength testing of specimens. In this research, the use of isothermal conduction calorimetry for the determination of apparent activation energy is investigated and compared with the traditional, standardized method. Several computational methods are used for the calculation and comparison of the apparent activation energy of cementitious materials using the isothermal conduction calorimetry testing. Accordingly, several techniques are used for the comparison of the apparent activation energy using compressive strength methods.

INTRODUCTION

The calculation of maturity and equivalent age of cementitious materials requires the time-temperature history in order to calculate rate of reaction (k) and apparent activation energy (E_a).¹ The calculation of equivalent age by the Arrhenius equation has been the equation most often used to describe the equivalent time per standardized test methods² and a large portion of available research. The Arrhenius equation as proposed by Freisleben-Hansen and Pederson³ is used to calculate equivalent time per equation 1.

$$t_e = \int_0^t e^{\frac{Q}{R} \left(\frac{1}{273+T_s} - \frac{1}{273+T_r} \right)} dt \quad (1)$$

Where:

t_e = equivalent age at a specified temperature
Q = activation energy divided by the universal gas constant

T_a = average temperature (°C) of the concrete during the time interval dt

T_r = Specified temperature, (typically 23° C US, 20° C Europe)

Per the standardized test method⁴, the calculation of maturity and equivalent age the testing of mortar cubes and concrete cylinders is conducted at early ages under isothermal conditions.

A hyperbolic equation is used to model the compressive strength evolution per equation 2.

$$S = S_u \frac{k(t-t_0)}{1+k(t-t_0)} \quad (2)$$

Where:
S = compressive strength (MPa)
t = test age (hours)
 S_u = limiting strength / ultimate strength
 t_0 = age when strength development is assumed to begin (hours)
k = rate constant

S_u , k and t_0 can be calculated using a best fit, curve fitting software.

The apparent activation energy is calculated by plotting the natural logarithm of the "k" values by the reciprocal of the absolute temperature.

BACKGROUND

Newer models have been proposed for the calculation E_a based on compressive strength testing which use an exponential equation to model compressive strength evolution over time as per equation 3⁴.

$$S = S_u \cdot \exp\left(-\frac{\tau}{t_e}^\beta\right) \quad (3)$$

Where:

S_u = ultimate strength

t_e = test age (hours)

β = shape constant

τ = time constant for strength prediction

Newer models have been proposed for the calculation of E_a based on isothermal calorimetry testing^{5,6}.

The development of a commercially available isothermal conduction calorimeter for small cementitious specimens (4-20g) has resulted in the development of models which calculate degree of hydration based isothermal conduction calorimetry testing results⁹ as shown by equation 4⁷.

$$\alpha = \alpha_u \cdot \exp\left(-\left(\frac{\tau}{t}\right)^\beta\right) \quad (4)$$

Where:

α = degree of hydration

t = test age (hours)

α_u = degree of ultimate hydration

β = shape constant

τ = time constant for strength prediction

Via the use of the isothermal calorimetry, and equation 3, the E_a is determined by equation 5, which is essentially the slope of the $-\ln(t)$ vs. $1/\text{Temperature}$ ⁷.

$$E_a = \frac{R \ln\left(\frac{\tau_{ref}}{\tau_c}\right)}{\left(\frac{1}{T_{ref}} - \frac{1}{T_c}\right)} \quad (5)$$

Where:

τ_{ref} = hydration time parameter at reference temperature

τ_c = is reference temperature

OBJECTIVES

The objectives of this research include modeling the E_a and prediction of physical behavior of cementitious materials using the following methods:
-ASTM 1074 (hyperbolic method) using compressive strength of cubes
-Exponential method using compressive strength of cubes
-Exponential method using isothermal conduction calorimetry

MATERIALS

Table 1. Mixture Design Properties for Specimens *

Material	Mix 1 100% Portland Cement (lb/cu ft)	Mix 2 50% Portland-50% Slag (lb/cu ft)	Mix 3 65% Portland- 35% Fly ash (lb/cu ft)	Mix 4 50-30-20 Normal (lb/cu ft)
Cement	681	341	443	341
Fly ash	0	0	238	136
GGSF Slag	0	341	0	204
Water	341	341	341	341
Fine Agg.	1095	1088	1036	1050
Coarse Agg.	1650	1668	1660	1650

*Coarse aggregate was not used for testing for Ea. Fine aggregate was not used in for isothermal conduction calorimetry testing.

RESULTS

The models for the prediction of mortar strength using the hyperbolic vs. exponential:

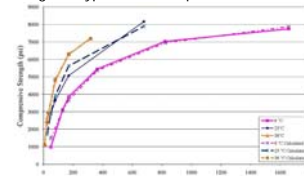


Figure 1. ASTM 1074 / hyperbolic calculated vs. measured compressive strength

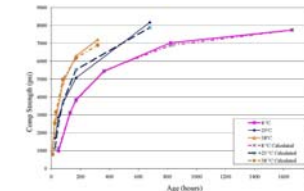


Figure 2. Exponential calculated vs. measured compressive strength

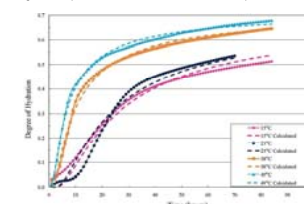


Figure 3. Degree of hydration measured vs. calculated per isothermal conduction calorimetry

Table 1. Comparison of E_a Calculation Methods

Mix Name	Compressive strength of mortar cubes		Isothermal Testing
	Hyperbolic (ASTM 1074)	Exponential	Exponential
Mix 1	35642	37401	34235
Mix 2	33688	39932	50400
Mix 3	25757	20643	32982
Mix 4	30013	21158	37330

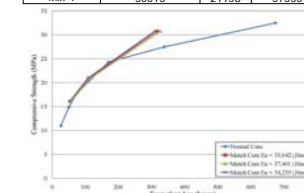


Figure 4. Compressive strength vs. equivalent age normal vs. match cure at elevated temperatures (Mix 1 - 100% portland cement)

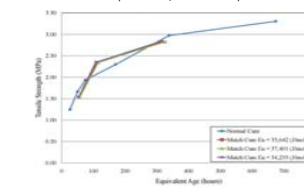


Figure 5. Tensile strength vs. equivalent age normal vs. match cure at elevated temperatures (Mix 1 - 100% portland cement)

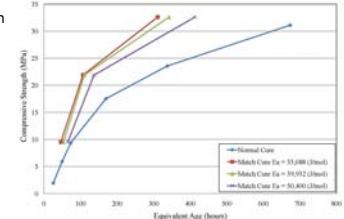


Figure 6. Compressive strength vs. equivalent age normal vs. match cure at elevated temperatures (Mix 2 - 50% portland - 50% Slag)

The variation of E_a does is not large regardless of calculation / testing method for concrete composed of Portland cement only. The variation of E_a is significant based on test calculation / testing method in concrete with large replacements of portland cement. The prediction of physical properties based on equivalent age are relatively constant regardless of E_a calculation method for concrete with portland cement alone. The prediction of physical properties of concrete containing large replacements of GGBF slag based on laboratory cured concrete underestimates strength properties.

CONCLUSIONS

The hyperbolic model is just as adequate as the exponential model for the prediction of early age physical properties of concrete¹ composed of Portland cement. More research is needed to verify the accuracy of prediction of concrete with large replacements of portland cement. Research should be conducted to establish links between E_a and phase morphology of cementitious materials.

REFERENCES

- Viviani, M., Glisic, B., Scrivner, K.L., Smith I.F.C., Equivalency Points: Prediction of concrete strength evolution in three days. *Cement and Concrete Research* (38) (8-9) (2008) 1070-1078
- ASTM C 1074-04 Standard Practice for Estimating Concrete Strength by the Maturity Method, ASTM International West Conshohocken, Pennsylvania: (2004) 8p
- Freisleben-Hansen, P., and Pederson, E.J., Maturity Computer for Controlling Curing and Hardening Concrete, *Nordisk Betong*, (1) (19)(1977) 19-34
- Schindler, A.K. *Concrete Hydration, Temperature Development, and Setting at Early Ages*. PhD Dissertation, University of Texas at Austin, Austin, TX. (2002)
- Ma, W., Sample, D., Martin, R., & Brown, P.W., Calorimetric Study of Cement Blends Containing Fly Ash, Silica Fume, and Slag at Elevated Temperatures, *Cement, Concrete, and Aggregates*, (16)(2)(1994) 93-99
- Broda, M., Wriquin, E., Duthoit, B., Conception of an Isothermal Calorimeter for Concrete - Determination of the Apparent Activation Energy Materials and Structures, (35)(2002) 389-394
- Poole, J.L., Riding, K.A., Follard, K.J., Juenger, M.C.G., & Schindler, A.K. (2007a). Methods for Calculating Activation Energy for Portland Cement. *ACI Materials Journal*, (104)(11)(2007)303-311

ACKNOWLEDGEMENTS

University of Florida Department of Civil and Coastal Engineering
Florida Department of Transportation State Materials Office:
Charles Ishbe, Richard DeLorenzo Toby Dillow, Alfred Camps
Jonathan L. Poole, WJE Dallas, TX
Jennifer L. Clark, University of Florida